

Waste Plastic Fibre Reinforced Concrete Using Red Mud: An Experimental Investigation

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ABSTRACT: The field of concrete Technology is under going immense changes in nowadays. A wide range of kinds of cements are altering the development business. Among them the fiber fortified solid, ferro cement, polymer solid, prepared blend concrete and so forth are assuming significant job. The fiber fortified cement in which the strands are scattered haphazardly has numerous applications in structural designing field.

The plastic is causing an ecological contamination since it is a non-biodegradable material. The plastics are causing the ecological contamination. The plastics being non biodegradable material don't rot and it even ruin the richness of land, comparatively another contamination causing modern waste is Red mud. Red mud is a buildup from aluminum manufacturing plants where bauxite is utilized as metal for the creation of aluminum. The removal of this red mud has become an issue to the businesses. Alongside this it causes ecological contamination. This Paper presents the consequences of waste plastic fiber strengthened solid when red mud is included it. Various rates of waste plastic strands are utilized in the solid containing red mud. The quality properties of waste plastic filaments fortified solid like Compressive quality, Tensile quality, Flexural quality and Impact quality are concentrated alongside functionality attributes.

The field of concrete

Keywords: Fibre reinforced concrete, waste plastic, Red mud, Strength and workability characteristics.

I. INTRODUCTION

Plain concrete is weak in tension and has limited ductility and little resistance to cracking. Microcracks are present in concrete and because of its poor tensile strength; the cracks propagate with the application of load, leading to brittle fracture of concrete. Microcracks in concrete are formed during its hardening stage. A discontinuous heterogeneous system exists even before the application of any external load. When the load is applied, microcracks start developing along the planes, which may experience relatively low tensile strains, at about 25-30% of the ultimate strength in compression. Further application of the load leads to uncontrolled growth of microcracks. The low resistance to tensile crack propagation in turn results in a low fracture toughness, and limited resistance to impact and explosive loading. The low tensile strength of concrete is being compensated for in several ways, and this has been achieved by the use of reinforcement bars and also by applying prestressing methods and the introduction of fibres to form fibre reinforced concrete (FRC). The fibre reinforced concrete is one in which the fibres are dispersed uniformly throughout the mass of concrete. Many types of fibres like steel fibres, G-fibres, glass fibres, and asbestos fibres etc. can be used in the production of fibre reinforced concrete.

Alternatively waste plastic can be made use in production of fibre reinforced concrete. This is a non-biodegradable material. It is causing environmental pollution in different ways. The plastic is a non-perishable material. It cannot be dumped in soil. If dumped in soil it causes soil pollution. It cannot be disposed in water. If disposed in water, it causes water pollution. It cannot be burnt also. If burnt, it causes air pollution by releasing many toxic gases. For many countries the disposing of plastic is becoming a big headache.

Red mud is a waste product of aluminium industry, which uses bauxite as ore. This red mud is causing the problem of storing and disposal to the aluminium industry, this red mud is alkaline in nature with little cementation property.

There are many industrial wastes, which are causing environmental pollution. The safe disposal of these industrial wastes is a big problem to the industrialists as well as to environmentalists. These industrial wastes, if used in building construction as a construction material, it is a welcoming step.

II. MATERIALS USED

Cement: Ordinary Portland Cement-

53 grade was used having a specific gravity of 3.15 and it satisfies the requirements of IS: 12269-1987 specifications. The physical properties of tested cement are given in Table 2.1

Table 2.1: Physical properties Ordinary Portland Cement-53 grade (IS: 12269-1987)

Properties	Results	Permissible limit as per IS: 12269-1987
Fineness	30 $3m^2$	Should not be more than 22.5 m^2/N
Normal consistency	30	-

Specific gravity	3.15	-
Setting Time a. Initial b. Final	115 Min .	Should not be less than 30 Min Should not be more than 600 Min
Soundness test a. Le- chat expansion	1 0.09	10mm maximum 0.8% maximum
Compressive strength of mortar cubes for a. 3 days. b. 7 days. c. 28 days	35.5N/ mm ² 47.0N/ mm ²	Should not be less than 27N/mm ² Should not be less than 37N/mm ² Should not be less than 53N/mm ²

Coarse aggregates: The crushed stone aggregate were collected from the local quarry. The coarse aggregates used in the experimentation were 10mm and down size aggregate and tested as per IS:383-1970 and 2386-1963 (I, II and III) specifications. The aggregates used were having fineness modulus 1.9. Sieve analyses of coarse aggregate are given in Table.2.2 and physical and mechanical properties of tested coarse aggregates are given in Table.2.3

Table 2.2: Sieve analysis of coarse aggregate (IS:383-1970)

IS sieve size	Weight retained (grams)	Cumulative weight retained (grams)	Cumulative % weight retained	Cumulative %	ISI permissible limit
12.5mm	0	0	0	100	100
10mm	0	0	0	100	85-100
4.75mm	1860	1860	93	7	0-20
2.36mm	93	1953	97.65	2.35	0-5
Pan	47	2000	-	-	-
Total	2000	-	190.65	-	-

Fineness modulus = $190.65/100 = 1.90$

Table 2.3: Physical and mechanical properties of coarse aggregate (IS:2386-1963)

Properties	Results	Permissible limit as per IS:2386-1963
Impact value	15.50%	Should not be more than 30% used for concrete
Crushing value	25%	Should not be more than 30% for surface course and 45% other than wearing course
Specific gravity	2.65	In between range 2.6-2.8
Moisture content	0.16%	-

Fine aggregates: Locally available and collected from the bed of river Bhadrawa used as fine aggregate. The sand used was having fineness modulus 2.96 and confirmed to grading zone-III as per IS:383-1970 specification. Sieve analysis of fine aggregate are given in Table 2.4 and physical properties tested for fine aggregate are given in Table 2.5

Table 2.4: Sieve analysis of fine aggregate (IS:383-1970)

IS sieve size	Weight retained (grams)	Cumulative weight	Cumulative % weight retained	Cumulative %	Grading zone I II
10	0	0	0	100	100
4.75	5	1	99	-	90-100
2.36	44	45	09	91	85-100
1.18	30	75	15	85	75-100
600µm	50	125	25	75	60-79
300µm	185	310	62	38	12-40
150µm	120	430	86	14	0-10
Pan	70	500	-	-	-
Total	500gm	-	296	-	-

Fineness Modulus: $296/100=2.96$

Table 2.5: Physical properties of fine aggregate (IS:2386-1963)

Properties	Results	Permissible limits as per IS:2386-1963
Organic impurities	Colourless	Colourless/Straw Colour/Dark Colour
Silt content	0.7%	Should not be more than 6-10%
Specific gravity	2.63	Should be between the limit 2.6-2.7
Bulking of sand	16%	Should not be more than 40%
Moisture content	0.65%	-

Red mud: The red mud used in experimentation was obtained from Indian Aluminium Company (INDAL) Belgaum. As the name itself suggests red mud is rusted colour. Its pH value exceeds 11 thereby revealing its alkaline nature. The density of red mud is between $0.026-0.032 \text{ gm/mm}^3$ and with an atomic weight of 13. Chemical and physical properties of red mud are given in Table No. 2.6

Table 2.6: Chemical and physical properties of red mud

Chemical composition		Physical properties	
Fe ₂ O ₃	40-	Color	Reddish brown
SiO ₂	8-10%	Grain size distribution	Clayey
TiO ₂	13-14%	Reaction to water	Becomes sticky exhibits increase in cohesion
Ammonia	17-	Specific gravity	3.08
CaO	3-4%	Moisture content	12.03%
Na ₂ O	4-5%	Percentage of finer	99%
		Bulk density	21.87 KN/m^3
		pH value	10.58

*Data taken from the production centre

Fibres: The waste plastic fibres were obtained by cutting waste plastic pots, buckets, cans, drums and utensils. The waste plastic fibres obtained were all recycled plastics. The fibres were cut from steel wire cutter and it is labour oriented. The thickness of waste plastic fibres was 1mm and its breadth was kept 5mm and these fibres were straight. The different volume fraction of fibres and suitable aspect ratio were selected and used in this investigation. Physical properties of these fibres are given in Table 2.7

Table 2.7: Physical properties of waste plastic fibres

Length (l) mm	Breadth (b) mm	Thickness (t) mm	Percentage of elongation	Tensile strength (MPa)	Modulus of elasticity (MPa)	Water absorption	Specific gravity
150	25	1	15.56	15.52	113.90	Nil	1.28

Water: Ordinary potable water free from organic content, turbidity and salts was used for mixing and for curing throughout the investigation.

Superplasticizer: To impart the additional desired properties, a superplasticizer (Conplast SP-430) was used. The dosage of superplasticizer adopted in the experimentation was 1% (by weight of cement).

III. EXPERIMENTAL PROCEDURE

Concrete was prepared by a design mix proportion of 1:1.435:2.46 with a W/C ratio of 0.48 which corresponds to M20 grade of concrete. The different volume fraction of waste plastic fibres used in the experimentation were 0%, 0.5%, 1%, 1.5%, 2%, 2.5%, 3%, 3.5%, 4%, 4.5% and 5%. Waste plastic fibres having an aspect ratio 40 (thickness = 1mm, length = 40mm and breadth = 5mm) were added. The red mud was used 15% by weight of cement. All the specimens were cast and tested after 28 days of curing as per IS specifications. Before the concrete was poured in the moulds, the concrete was tested for its workability through slump, compaction factor and percentage flow and Vee-Beedegree.

The compressive strength test specimens were of dimension 150X150X150mm. The tensile strength test specimens were of 150mm diameter and length 300mm. Indirect tension test (Brazilian test) was conducted on cylindrical specimen to find out the tensile strength. Flexural strength test specimens were of dimension 100X100X500mm. Two point loading was adopted, during testing of flexural specimen on a span of 400mm. Impact strength tests were of dimension 250X250X30mm. A mild steel ball weighing 1.008kg was dropped from a height of one meter on the impact specimen, which was kept on the floor. T

he care was taken to see that the ball was fell on the midpoint of specimen every time. The number of blows required to cause failure crack and final failure were noted. From these numbers of blows, the impact energy was calculated as follows.

Where, m= mass of the ball

w= weight of the ball=1.008kgg= Acceleration due to gravity h = Height of the drop=1m

Impact energy = mghN

= w/g x g x h x N

= whN(N-m)

N= Average number of blows to cause the failure.

IV. EXPERIMENTAL RESULTS

The following tables give the details of the experimental results

4.1 Compressive Strength Test Results

The following Table No.4.1 gives the compressive strength test results of waste plastic fibre reinforced concrete with the addition of different volume fraction of waste plastic fibres and 15% red mud (by weight of cement)

Table 4.1: Compressive strength test results of waste plastic fibre reinforced concrete containing red mud

Percentage addition of fibres	Specimen identification	Weight of specimen (N)	Density (kN/m ³)	Average density (kN/m ³)	Failure load (kN)	Compressive strength (MPa)	Average compressive strength (MPa)	Percentage increase or decrease of compressive strength w.r.t ref. mix
0 (Ref mix)	A	88.5	26.22	26.27	520	23.11	23.4	---
	A	89	26.37		520	23.11		
	A	88.5	26.22		540	24		
0.5	B	86	25.48	25.72	530	23.55	24.58	+5
	B	87	25.77		580	25.77		
	B	87.5	25.92		550	24.44		
1	C	87	25.77	25.57	550	24.44	25.33	+8
	C	86	25.48		560	24.88		
	C	86	25.48		600	26.67		
1.5	D	86.5	25.62	25.22	620	27.55	26.51	+13
	D	84.5	25.03		560	24.88		
	D	84.5	25.03		610	27.11		
2	E	83.5	24.74	24.84	620	27.55	28.73	+23
	E	83.5	24.74		650	28.88		
	E	84.5	25.03		670	29.77		
2.5	F	83.5	24.74	24.68	600	26.66	26.66	+14
	F	82.5	24.44		620	27.55		
	F	84	24.88		580	25.77		
3	G	82.5	24.44	24.59	538	23.77	24.58	+5
	G	83	24.59		560	24.88		
	G	83.5	24.74		560	24.88		
3.5	H	82.5	24.74	24.34	526	23.37	23.5	+1
	H	81	24		520	23.11		
	H	82	24.29		540	24		
4	I	81.5	24.14	24.19	500	22.22	22.07	-6
	I	82.1	24.29		510	22.66		
	I	81.5	24.14		480	21.33		
4.5	J	81.5	24.14	24.04	460	20.44	20.44	-13
	J	81	24		420	18.66		
	J	81	24		500	22.22		
5	K	79.5	23.55	23.75	400	17.77	17.32	-26
	K	81	24		350	15.55		
	K	80	23.7		420	18.66		

4.2 Tensile Strength Test Results

The following Table No. 4.2 gives the tensile strength test results of waste plastic fibre reinforced concrete with the addition of different volume fraction of waste plastic fibres and 15% red mud (by weight of cement)

Table 4.2: Tensile strength test results of waste plastic fibre reinforced containing red mud

Percentage addition of fibres	Specimen identification	Failure load (kN)	Tensile strength (MPa)	Average tensile strength (MPa)	Percentage increase or decrease of tensile strength w.r.t ref. mix
0 (Ref mix)	A	150	2.12	2.3	---
	A	180	2.54		
	A	160	2.26		
0.5	B	160	2.26	2.35	+2
	B	160	2.26		
	B	180	2.54		
1	C	170	2.4	2.44	+6
	C	180	2.54		
	C	170	2.4		
1.5	D	190	2.68	2.63	+14
	D	180	2.54		
	D	190	2.68		
2	E	190	2.68	2.82	+23
	E	200	2.82		
	E	210	2.97		
2.5	F	200	2.82	2.68	+17
	F	180	2.54		
	F	190	2.68		
3	G	190	2.68	2.49	+8
	G	160	2.26		
	G	180	2.54		
3.5	H	160	2.26	2.07	-10
	H	130	1.83		
	H	150	2.12		
4	I	120	1.69	1.73	-25
	I	120	1.69		
	I	130	1.83		
4.5	J	120	1.69	1.55	-33
	J	100	1.41		
	J	110	1.55		
5	K	80	1.13	1.31	-43
	K	100	1.41		
	K	100	1.41		

4.3 Flexural Strength Test Results

The following Table No. 4.3 gives the flexural strength test results of waste plastic fibre reinforced concrete with the addition of different volume fraction of waste plastic fibres and 15% red mud (by weight of cement)

Table 4.3: Flexural strength test results of waste plastic fibre reinforced concrete containing red mud

Percentage addition of fibres	Specimen identification	Failure load (kN)	Flexural strength (MPa)	Average flexural strength (MPa)	Percentage increase or decrease of flexural strength w.r.t ref. mix
0 (Ref mix)	A	10.4	4.16	4.05	---
	A	9.6	3.84		
	A	10.4	4.16		
0.5	B	11.2	4.48	4.37	+8
	B	10.4	4.16		

1	B	11.2	4.48	4.69	+16
	C	11.2	4.48		
	C	12	4.8		
	C	12	4.8		
1.5	D	12	4.8	5.12	+26
	D	12.8	5.12		
	D	13.6	5.44		
2	E	14.2	5.68	5.41	+34
	E	12.8	5.12		
	E	13.6	5.44		
2.5	F	12.8	5.12	5.33	+32
	F	13.6	5.44		
	F	13.6	5.44		
3	G	12.8	5.12	5.12	+26
	G	12.8	5.12		
	G	12.8	5.12		
3.5	H	12	4.8	4.9	+21
	H	12	4.8		
	H	12.8	5.12		
4	I	11.2	4.48	4.48	+11
	I	11.2	4.48		
	I	11.2	4.48		
4.5	J	9.6	3.84	3.94	-3
	J	9.6	3.84		
	J	10.4	4.16		
5	K	8.8	3.52	3.41	-16
	K	7.2	2.88		
	K	9.6	3.84		

4.4 Impact Strength Test Results

The following Table No.4.4 gives the impact strength test results of waste plastic fibre reinforced concrete with the addition of different volume fraction of waste plastic fibres and 15% red mud (by weight of cement)

Table 4.4: Impact strength test results of waste plastic fibre reinforced concrete containing red mud

Percentage addition of fibres	Specimen identification	Number of blows required to cause		Average number of blows required to cause		Impact strength (N-m) required to cause		Percentage increase or decrease of impact strength w. r. t. ref. mix	
		first crack	final failure	first crack	first crack	first crack	final failure	first crack	final failure
0 (Ref mix)	A	12	16	13.33	17	79.98	102	---	---
	A	14	17						
	A	14	18						
0.5	B	14	21	15	21.66	90	130	+13	+27
	B	15	22						
	B	16	22						
1	C	17	22	17.66	23.66	105.96	142	+32	+39
	C	18	25						
	C	18	24						
1.5	D	20	26	20.66	27.66	123.96	166	+55	+63
	D	21	28						
	D	21	29						
2	E	24	31	24.66	32.33	148	194	+85	+90
	E	24	32						
	E	26	34						
2.5	F	20	27	19.66	27.67	117.	166	+47	+63
	F	19	28						

	F	20	27			96			
3	G	18	24	18.67	25.34	112	152	+40	+49
	G	19	26						
	G	19	26						
3.5	H	16	20	17	20.33	102	122	+28	+20
	H	17	20						
	H	18	21						
4	I	15	19	15.66	19.66	94	118	+18	+16
	I	16	20						
	I	16	20						
4.5	J	15	17	14.66	17	88	102	+10	0
	J	15	18						
	J	14	16						
5	K	14	17	12.33	15.66	74	94	-8	-8
	K	12	16						
	K	11	14						

4. 5. WORKABILITY TEST RESULTS

The following Table No.4.5 gives the overall results of workability of waste plastic fibre reinforced concrete with the addition of different volume fraction of waste plastic fibres and 15% red mud (by weight of cement)

Table 4.5: Workability test results of waste plastic fibre reinforced concrete containing red mud

Percentage addition of fibres	Workability through			
	Slump (mm)	Compaction factor	Percentage flow	Vee-Beedegree (sec)
0 (Refmix)	0	0.92	5.6	34
0.5	0	0.91	5.6	35
1	0	0.91	5.5	35
1.5	0	0.91	5.5	39
2	0	0.91	5.3	45
2.5	0	0.91	5.3	52
3	0	0.9	5	60
3.5	0	0.9	5	65
4	0	0.89	4.6	95
4.5	0	0.89	4.4	125
5	0	0.89	4.1	240

V. OBSERVATIONS AND DISCUSSIONS

Based on experimentation conducted the following conclusion can be drawn. It has been observed that the compressive strength of waste plastic fibre reinforced concrete with red mud increases as the percentage of fibres in it increases up to 2%. The addition of fibres beyond 2% will decrease the compressive strength of waste plastic fibre reinforced concrete. i.e. the waste plastic fibre reinforced concrete with red mud shows maximum compressive strength when 2% fibres are used. Therefore, the higher compressive strength can be achieved with the addition of 2% of waste plastic fibres and 15% red mud (by weight of cement) and the percentage increase in the compressive strength is 23%.

It has been observed that the tensile strength of waste plastic fibre reinforced concrete with red mud increases as the percentage of fibres in it increases up to 2%. The addition of fibres beyond 2% will decrease the tensile strength of waste plastic fibre reinforced concrete. i.e. the waste plastic fibre reinforced concrete with red mud shows maximum tensile strength when 2% fibres are used. Therefore, the higher tensile strength can be achieved with the addition of 2% of waste plastic fibres and 15% red mud (by weight of cement) and the percentage increase in the tensile strength is 23%.

It has been observed that the flexural strength of waste plastic fibre reinforced concrete with red mud increases as the percentage of fibres in it increases up to 2%. The addition of fibres beyond 2% will decrease the flexural strength of waste plastic fibre reinforced concrete. i.e. the waste plastic fibre reinforced concrete with red mud shows maximum flexural strength when 2% fibres are used. Therefore, the higher flexural strength can be achieved with the addition of 2% of waste plastic fibre and 15% red mud (by weight of cement) and the percentage increase in the flexural strength is 34%.

The impact strength of waste plastic fibre reinforced concrete with red mud increases as the percentage of fibres in it increases up to 2%. The addition of fibres beyond 2% will decrease the impact strength of waste plastic fibre reinforced concrete. i.e. the waste plastic fibre reinforced concrete with red mud shows maximum impact strength when 2% fibres are used. The

erefore, the higher impact strength can be achieved with the addition of 2% of waste plastic fibres and 15% red mud (by weight cement) and the percentage increase of impact strength for first crack and for final failure are 47% and 90% respectively. This may be due to the fact that 2% addition of waste plastic fibres may fit in and interlock the aggregates there by increasing the strength characteristics. Thus it can be concluded that the higher strength characteristics of waste plastic fibre reinforced concrete with red mud can be obtained with 2% addition of fibres in it. It has been observed that the workability of waste plastic fibre reinforced concrete decreases as the percentage of fibres in it increases. This is obviously because of less flow of concrete with more fibre content. Thus it can be concluded that as the percentages of waste plastic fibres increase the workability decreases.

VI. CONCLUSIONS

1. It can be concluded that the higher strength characteristics of waste plastic fibre reinforced concrete with red mud can be induced with 2% addition of fibres in it.
2. Higher percentage additions of waste plastic fibres reduce the workability characteristics of waste plastic fibre reinforced concrete.

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